

Evaluation Report

CPER - 101

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Revised: n/a

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Evaluation Subject:

Equinox Extruded Aluminum Rotatable Louvers for Shade Structures

1.0 Scope:

Product Category: Structural Component

Subcategory: Roof Deck

This report shows this product complies with the 2010 Florida Building Code 1504.1, 1504.9 and Section 1609. This rotating louver system has been tested in accordance with TAS 202.

2.0 Uses: Equinox Aluminum Louvers are metal rotatable roof panels to be used for outdoor recreational living, carports or exterior commercial use.

3.0 Description:

3.1 Louver: The 6063T5 aluminum louver is an extruded shape shown in Detail F1 and in Appendix E of the test report cited in 6.1.

3.2 End Cap: A380 cast aluminum end cap shown in Detail F5.

3.3 Mounting Rail: 6061T6 aluminum extruded mounting rail shown in Detail F3.

3.4: Header: 6061T6 0.125"x2"x8" aluminum extruded header shown in Detail F2. Other headers may be used but equivalent fastening of the mounting rail must be demonstrated.

3.5 Fasteners: #12 and #14 sheet metal screws.

4.0 Design and Installation:

4.1 Design: Use Table 1 in to determine allowable span for 20 psf live loads and appropriate wind condition.

5.0 Conditions of Use:

5.1 Equinox Louvers are for outside recreational living spaces (patio covers), carports and exterior commercial spaces.

5.2 Figures 1609A, B and C will determine relevant local wind speed

5.3 Supporting structure shall be justified by a Florida design professional.

5.4. Louvers are permitted in HVHZ

6.0 Evidence Submitted:

6.1 ATI Test Report # D7097.01-109-18r2

6.2 Louver span calculations

6.3 Extruded Aluminum Louver Die Drawing

6.4 Cast Aluminum End Cap die drawing

Table 1 LOUVER SPANS FOR COMMERCIAL AND PATIO STRUCTURES for 2010 FBC

Roof Design Load (psf)	Equinox Extruded Aluminum Louvers	Wind Speed (mph)						
		120	130	140	150	160	170	180
Allowable spans for Group R-3 occupancy structures								
20	Wind Exposure B	13'-3"	13'-3"	13'-3"	13'-3"	13'-3"	0'-0"	0'-0"
	Wind Exposure C	13'-3"	13'-3"	13'-3"	13'-3"	13'-3"	11'-6"	11'-1"
	Wind Exposure D	13'-3"	13'-3"	13'-3"	12'-11"	12'-4"	10'-10"	10'-5"
Allowable Spans for all other structures								
20	Wind Exposure B	13'-3"	13'-3"	13'-3"	13'-3"	13'-3"	0'-0"	0'-0"
	Wind Exposure C	13'-3"	13'-3"	13'-3"	13'-3"	13'-3"	8'-0"	7'-8"
	Wind Exposure D	13'-3"	13'-3"	13'-3"	12'-11"	12'-4"	7'-6"	7'-2"

Notes

- 1 Live/Wind spans are limited by deflection of 60 as permitted by footnote a in Table 1604.3 2010 FBC
- 2 Deflections for 170 and 180 mph winds for R-3 structures are limited by L/80 as per 2010 FBC 1615.3.1 #8
- 3 Deflections for 170 and 180 mph winds for non R-3 structures are limited by L/240 as per 2010 FBC 1615.3.1 #2
- 4 The weight of the louver is 1.9 plf or 2.9 psf.
- 5 These tables are only to be used for exterior shade structures such as patio covers, carports or outdoor dining areas.
- 6 Topographic Factor, Kzt, of 1 is assumed (ASCE7-10 Fig 26.8-1)
- 7 Wind pressures assume a maximum mean roof height of 15'
- 8 Span is the Louver length plus the aluminum mounting rail on both sides, this is usually the inside to inside dimension of the supporting headers
- 9 Panels are allowed in HVHZ
- 10 Exposure B is not allowed in HVHZ

This worksheet determines the allowable spans for a panel using raw data from transverse load test

Louver Specs Panel := "Equinox Extruded Louver"

Width := 8-in DL := 1.918-plf per die drawing Elouwer := 10100-ksi t := 0.065-in

I := 0.437-in⁴ determined in "Patio Cover Blade Allowable Shear and Bending" 3/27/2013 by Randy Kissell

Define user functions

maxi(amaxi, bmaxi) := if(amaxi > bmaxi, amaxi, bmaxi) Chooses the max value between 2 numbers

mini(amaxi, bmaxi) := if(amaxi > bmaxi, bmaxi, amaxi) Chooses the min value between 2 numbers

Raw Testing Data from Architectural Testing Inc. D7097.01-109-18r2

Pres := $\begin{pmatrix} 10 & 10 & 5 \\ 20 & 20 & 10 \\ 30 & 30 & 15 \\ 40 & 35 & 20 \\ 50 & 40 & 25 \\ 60 & 45 & 30 \\ 70 & 50 & 35 \\ 80 & 55 & 40 \\ 90 & 60 & 40 \\ 100 & 65 & 40 \\ 110 & 70 & 40 \\ 120 & 75 & 40 \\ 130 & 80 & 40 \\ 140 & 85 & 40 \\ 150 & 85 & 40 \\ 160 & 85 & 40 \\ 170 & 85 & 40 \end{pmatrix}$ ·psf	$\Delta_{raw} := \begin{pmatrix} 0.17 & 0.77 & 0.85 \\ 0.33 & 1.52 & 1.59 \\ 0.49 & 2.20 & 2.26 \\ 0.65 & 2.53 & 2.9 \\ 0.82 & 2.85 & 3.49 \\ 0.98 & 3.11 & 3.93 \\ 1.13 & 3.43 & 4.52 \\ 1.29 & 3.75 & 5.05 \\ 1.44 & 4.01 & 5.05 \\ 1.59 & 4.24 & 5.05 \\ 1.75 & 4.49 & 5.05 \\ 1.9 & 4.73 & 5.05 \\ 2.06 & 5.04 & 5.05 \\ 2.24 & 5.34 & 5.05 \\ 2.4 & 5.34 & 5.05 \\ 2.62 & 5.34 & 5.05 \\ 2.8 & 5.34 & 5.05 \end{pmatrix}$ ·in	ndata := $\begin{pmatrix} 16 \\ 13 \\ 7 \end{pmatrix}$ max data point, after this number all data repeated i := 0..2 j := 0..16 i3 := 0..1 $L := \begin{pmatrix} 92 \\ 140 \\ 163 \end{pmatrix}$ ·in k _i := 0..ndata _i SF := 2 dcrit := 60 MaxSpan := max(L) = 13.583 ft
--	--	---

$$V := \frac{(Pres_{ndata_0,0} \cdot Width) \cdot L_0}{2 \cdot SF} = 217 \text{ lbf}$$

use short span to determine allowable shear

$$M := \frac{(Pres_{ndata_1,1} \cdot Width) \cdot (L_1)^2}{8 \cdot SF} = 482 \cdot \text{lbf} \cdot \text{ft}$$

use middle span to determine allowable moment

Determine Coefficient of Deflection: The coefficient, C, from the deflection equation

def = C wL⁴ / EI , for a single span, perfect pin and roller jointed ends this value is 5/384 or 0.013.

$$coeff_j := \Delta_{raw_j,2} \cdot \frac{Elouwer \cdot I}{(Pres_{j,2} \cdot Width) \cdot (L_2)^4}$$

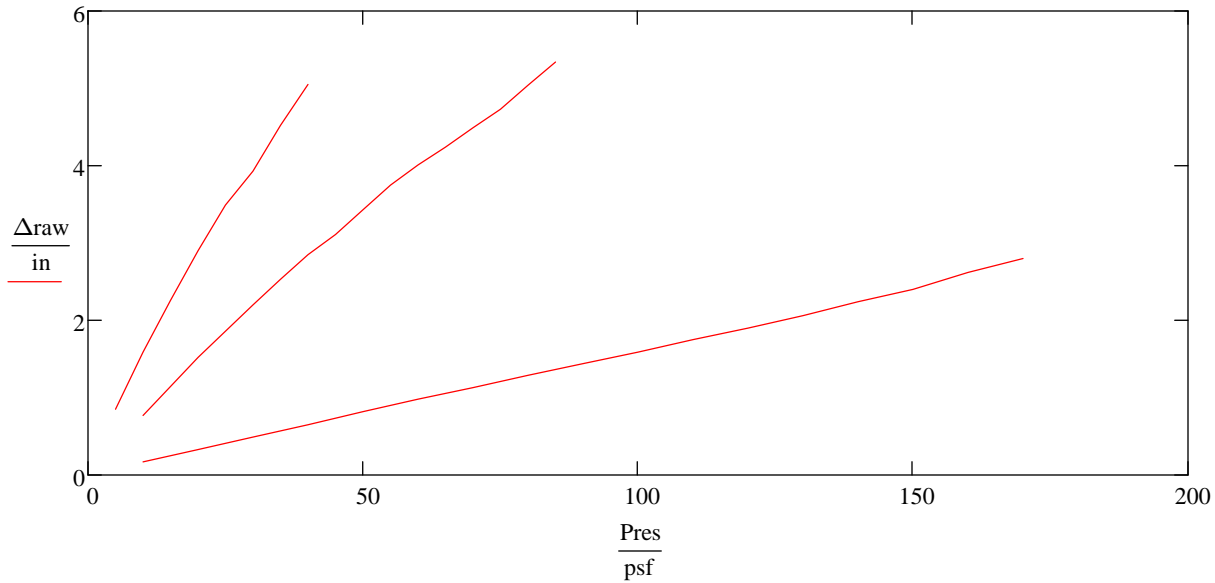
single span, pin jointed span

$$CoeffTheory := \frac{5}{384} = 0.01302$$

$$Coeff := \frac{\sum_{k1=0}^{ndata_2} coeff_{k1}}{ndata_2 + 1}$$

use long span to determine deflection coefficient

Coeff = 0.016 average coefficient CoeffTheory = 0.013



i0 := 0..ndata0 i1 := 0..ndata1 i2 := 0..ndata1

Define Loads

nf := 41 number of live loads

n := 0..nf - 1 live load counter Imp := 1.0 Importance factor, all structures are Type II

Load Combinations used is in accordance with IBC 2009 1605.3.1. Equation 16-9 and Equation 16-11 will control.

Design Loads

$$LL_n := \text{if}[n > 0, (10 + 1 \cdot n) \cdot \text{psf}, 10 \cdot \text{psf}] \cdot \text{Imp}$$

$$LL_{h0,n} := LL_n \quad \max(LL) = 50 \cdot \text{psf}$$

	0
0	10
1	11
2	12
3	13
4	14
5	15
6	16
7	17
8	18
9	19
10	20
11	...

LL =

5	15
---	----

 · psf

Determine Dead and Live loads per lineal foot

$$w_n := (LL_n) \cdot \text{Width} + DL \quad w_{LL_n} := LL_n \cdot \text{Width}$$

Determine max spans for all allowable stresses

Top in compression

$$L_{\text{shear}_n} := \frac{V}{0.5 \cdot w_n} \quad L_{\text{def}_n} := \sqrt[3]{\frac{E_{\text{louver}} \cdot I}{\text{Coeff} \cdot w_{LL_n} \cdot d_{\text{crit}}}} \quad L_{\text{bendT}_n} := \sqrt{\frac{8 \cdot M}{w_n}}$$

$$L_{\text{snow}_n} := \text{mini}(L_{\text{def}_n}, L_{\text{shear}_n}) \quad L_{\text{snow}_n} := \text{mini}(L_{\text{bendT}_n}, L_{\text{snow}_n}) \quad L_{\text{snow}_n} := \text{mini}(\text{MaxSpan}, L_{\text{snow}_n})$$

	0
0	16.8
1	16.3
2	15.8
3	15.4
4	15
5	14.7

Ldef =

5	14.7
---	------

 ft

	0
0	21.2
1	20.4
2	19.7
3	19.1
4	18.5
5	18

LbendT =

5	18
---	----

 ft

	0
0	51
1	47
2	44
3	41
4	39
5	36

Lshear =

5	36
---	----

 ft

6	14.4
7	14.1
8	13.8
9	13.6
10	13.3
11	...

6	17.5
7	17.1
8	16.6
9	16.3
10	15.9
11	...

6	35
7	33
8	31
9	30
10	28
11	...

	0
0	10
1	11
2	12
3	13
4	14
5	15
6	16
7	17
8	18
9	19
10	20
11	...

	0
0	13.6
1	13.6
2	13.6
3	13.6
4	13.6
5	13.6
6	13.6
7	13.6
8	13.6
9	13.6
10	13.3
11	...

LL =

Lsnow =

Max Spans Based on
Snow Loads for
Panel = "Equinox Extruded Louver"

Wind Loads are determined by procedure explained in ASCE 7-10 Section 30.8 and Figure 30.8-1.

$$nm := 7 \quad nw1 := 0..nm - 1 \quad nw2 := nm..2 \cdot nm - 1 \quad nw3 := 2 \cdot nm..3 \cdot nm - 1 \quad nw := 0..3 \cdot nm - 1$$

$$WS_0 := 120 \cdot \text{mph} \quad WS_1 := 135 \cdot \text{mph} \quad WS_2 := 140 \cdot \text{mph} \quad WS_3 := 150 \cdot \text{mph} \quad WS_4 := 160 \cdot \text{mph} \quad WS_5 := 170 \cdot \text{mph}$$

$$WS_6 := 180 \cdot \text{mph} \quad WSh_{0,nw1} := WS_{nw1} \quad WSh_{0,nw1+nm} := WS_{nw1} \quad WSh_{0,nw1+2 \cdot nm} := WS_{nw1}$$

$$\text{deflection limit} \quad d_{wind,nw,0} := \text{if}(WSh_{0,nw} > 169 \cdot \text{mph}, 80, 60)$$

$$d_{wind,nw,1} := \text{if}(WSh_{0,nw} > 169 \cdot \text{mph}, 240, 60)$$

Deflection limits change for HVHZ

Wind Directionality Factor Table 26.6-1 $K_d := 0.85$

ASCE7 Table 30.3-1 Exposures B and C (C&C)

$$K_zB := 0.7 \quad K_zC := 0.85 \quad K_zD := 1.03$$

ASCE7 Fig 26.8-1 Topographic factors are outside the scope of this report

$$K_{zt} := 1.0$$

$$WSh_{0,nw1+nm} := WS_{nw1}$$

ASCE7 30.3.2

$$q_{znw1} := 0.00256 \cdot K_zB \cdot K_{zt} \cdot K_d \cdot (WS_{nw1})^2 \cdot 14.959 \cdot \frac{\text{lb}}{\text{ft}^3} \cdot 0.6$$

14.959 lb/ft³ is a hidden constant used for unit conversion in the ASCE 7 equation 30.3-1 that MathCAD requires to be explicitly stated. I also use the 0.6 factor here for the Vult windspeeds

$$q_z = \begin{pmatrix} 13.2 \\ 16.7 \\ 17.9 \\ 20.6 \\ 23.4 \\ 26.4 \\ 29.6 \end{pmatrix} \cdot \text{psf}$$

$$q_{znw1+nm} := q_{znw1} \cdot \frac{K_zC}{K_zB} \quad q_{znw1+2 \cdot nm} := q_{znw1} \cdot \frac{K_zD}{K_zB}$$

added 0.6 factor (from Eqtn 16-12 and 13) here

$$\text{MinP} := 0.6 \cdot 16 \cdot \text{psf}$$

	0
0	13.2
1	16.7
2	17.9
3	20.6
4	23.4

ASCE7 26.9.1

$G := 0.85$

ASCE7 Fig 30.8-1 angle = 0 , Area > 4a^2

CN := 1.2

4	25.4
5	26.4
6	29.6
7	16
8	20.2
9	21.8
10	25
11	...

·psf

Added 0.6 (for Vult) into qz already. This requires me to use 0.7 instead of 0.42 wind load factor (for deflection) into these equations, Table 1604 footnote f.

$wW_{nw} := \max(qz_{nw} \cdot G \cdot CN, \text{MinP}) \cdot \text{Width} + DL$

$wW2_{nw} := \max(qz_{nw} \cdot G \cdot CN, \text{MinP}) \cdot 0.7 \cdot \text{Width}$

$Lwshear_{nw} := \frac{V}{0.5 \cdot wW_{nw}}$

$Lwdef_{nw,i3} := \sqrt[3]{\frac{\text{Elouwer} \cdot I}{\text{Coeff} \cdot wW2_{nw} \cdot dwind_{nw,i3}}}$

$LwbendT_{nw} := \sqrt{\frac{8 \cdot M}{wW_{nw}}}$

Top/bot in compression

Lwshear =

	0
0	40
1	33
2	31
3	27
4	24
5	22
6	20
7	34
8	28
9	26
10	23
11	20
12	18
13	16
14	29
15	23
16	22
17	19
18	17
19	15
20	14

ft

LwbendT =

	0
0	18.8
1	17.1
2	16.5
3	15.6
4	14.7
5	13.9
6	13.2
7	17.4
8	15.7
9	15.2
10	14.3
11	13.5
12	12.7
13	12.1
14	16
15	14.4
16	13.9
17	13.1
18	12.3
19	11.7
20	11.1

ft

Lwdef =

	0	1
0	17.1	17.1
1	15.8	15.8
2	15.5	15.5
3	14.8	14.8
4	14.2	14.2
5	12.3	8.6
6	11.9	8.2
7	16.1	16.1
8	14.9	14.9
9	14.5	14.5
10	13.8	13.8
11	13.3	13.3
12	11.6	8
13	11.1	7.7
14	15.1	15.1
15	13.9	13.9
16	13.6	13.6
17	13	13
18	12.4	12.4
19	10.9	7.5
20	10.4	7.2

ft

WS =

120
135
140
150
160
170
180

·mph

Wind Exposure

Exp B

WS =

120
135
140
150
160
170
180

·mph

Exp C

WS =

120
135
140
150
160
170
180

·mph

Exp D

These loading conditions are discussed in detail in the Design Forces: Wind Loads section

$$Lw_{nw,i3} := \text{mini}(Lw_{def_{nw,i3}}, Lw_{shear_{nw}})$$

$$Lw_{nw,i3} := \text{mini}(Lw_{bendT_{nw}}, Lw_{nw,i3})$$

$$Lw_{nw,i3} := \text{mini}(\text{MaxSpan}, Lw_{nw,i3})$$

Set $Lw = 0$ for Exposure B in $WS > 169$ mph (HVHZ)

$$Lw_{nw1,i3} := \text{if}(WS_{nw1} > 169 \cdot \text{mph}, 0, Lw_{nw1,i3})$$

$V = 217$ lbf allowable shear

$M = 482$ lbf·ft allowable moment

Coeff = 0.0162 deflection coefficient

Wind Rating
(mph)

Exp B

WS =

- 120
- 135
- 140
- 150
- 160
- 170
- 180

Exp C

WS =

- 120
- 135
- 140
- 150
- 160
- 170
- 180

Exp D

WS =

- 120
- 135
- 140
- 150
- 160
- 170
- 180

	0	1
0	13.6	13.6
1	13.6	13.6
2	13.6	13.6
3	13.6	13.6
4	13.6	13.6
5	0	0
6	0	0
7	13.6	13.6
8	13.6	13.6
9	13.6	13.6
10	13.6	13.6
11	13.3	13.3
12	11.6	8
13	11.1	7.7
14	13.6	13.6
15	13.6	13.6
16	13.6	13.6
17	13	13
18	12.3	12.3
19	10.9	7.5
20	10.4	7.2

Lw =

Max Spans Based on Wind Loads for Panel = "Equinox Extruded Louver"

$$L_{snowX_{0,n}} := L_{snow_n} \quad LwX_{i3,nw} := Lw_{nw,i3}$$

$L_{snowX} =$

	0	1	2	3	4	5	6	7	8	9	10	11	12	13	14
0	13.6	13.6	13.6	13.6	13.6	13.6	13.6	13.6	13.6	13.6	13.3	13.1	12.9	12.7	...

ft

$LwX =$

	0	1	2	3	4	5	6	7	8	9	10	11	12
0	13.6	13.6	13.6	13.6	13.6	0	0	13.6	13.6	13.6	13.6	13.3	11.6
1	13.6	13.6	13.6	13.6	13.6	0	0	13.6	13.6	13.6	13.6	13.3	...

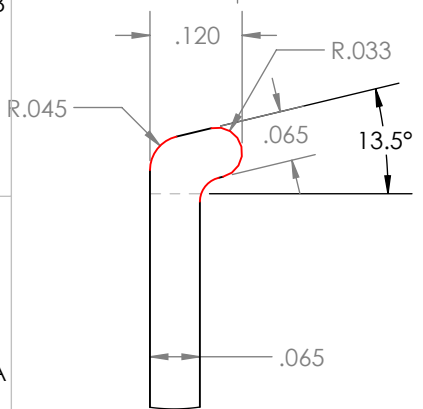
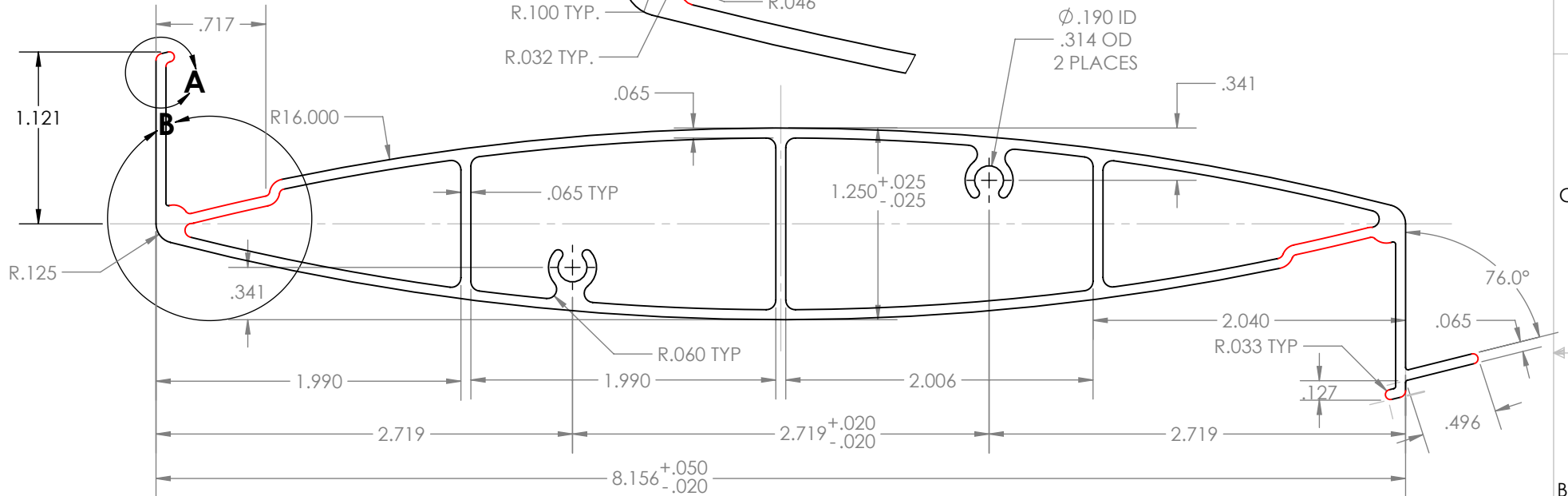
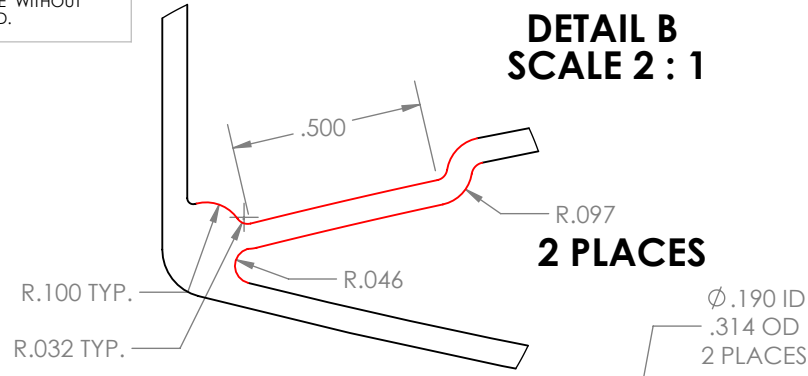
ft

$$\min(dwind) = 60 \quad \max(dwind) = 240$$

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**DETAIL B
SCALE 2 : 1**

2 PLACES



**DETAIL A
SCALE 4 : 1
2 PLACES**

**NOTE: LENGTH TO BE $+0.125$
 -0.000**

ASSEMBLY: NTS

UNLESS OTHERWISE SPECIFIED
DIMENSIONS ARE IN INCHES
TOLERANCES ARE:
FRACTIONS

**DECIMALS $+ .002$
 $- .002$ "**

ANGLES

**MATERIAL
ALUMINUM
.065 WALL**

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APPROVALS	DATE
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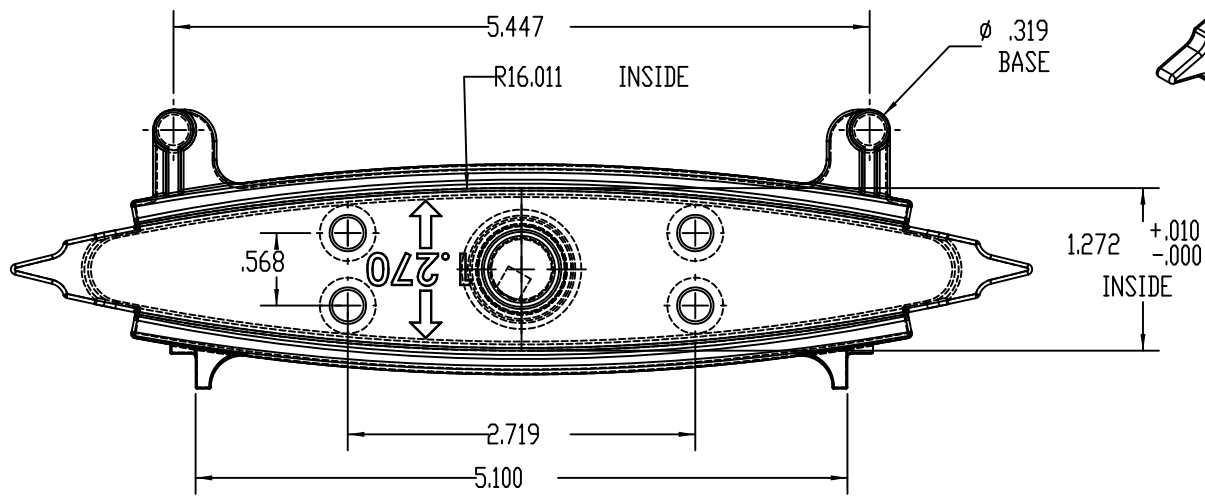
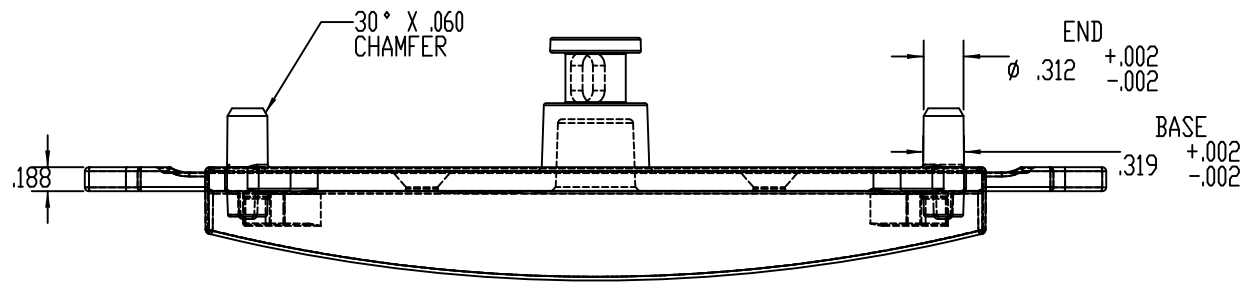
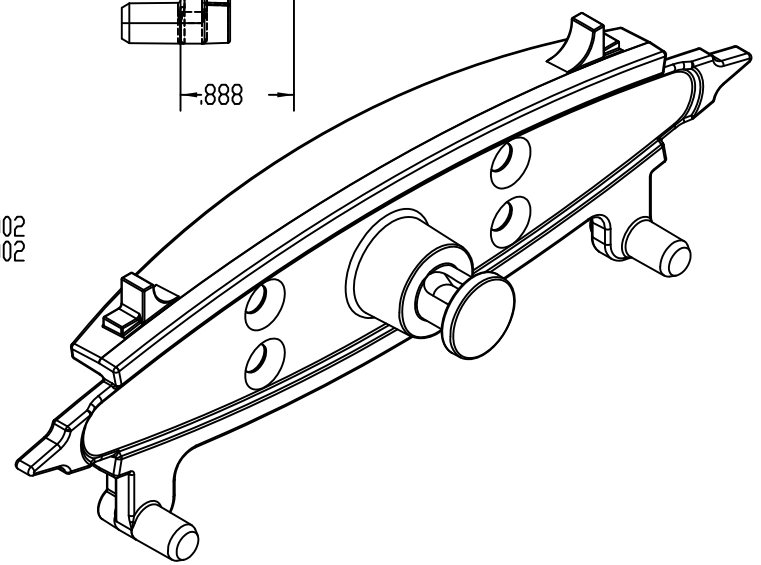
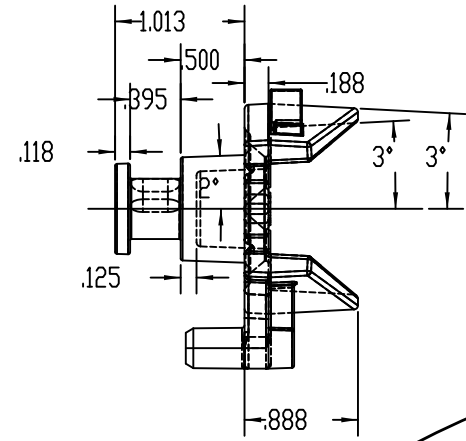
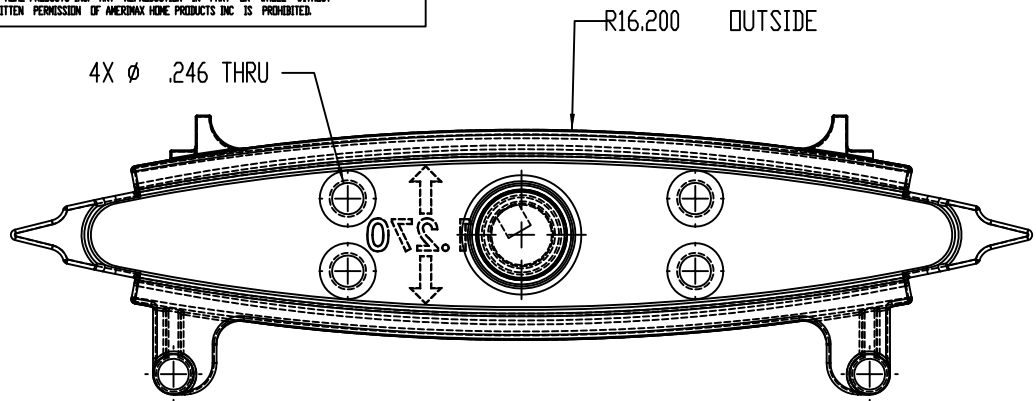
SIZE CAD FILE:
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QUAL ENG

AMERIMAX
HOME PRODUCTS INC
**PATIO COVER BLADE
.065 WALL**

SCALE **1:1** DWG. NO. **Sheet1** SHEET **1** OF **1**
3/4/2014

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	APPROVALS	DATE	
DECIMALS ±.005	DRWN JG	8-26-13	
ANGLES	CHECKED		
MATERIAL ALUMINUM	SIZE CAD FILE		
DO NOT SCALE DRAWING	SCALE 1:1	DATE 10/15/2013	SHEET 1 OF 1